

The Continuous Strength Method of Cold-formed Stainless Steel Equal-leg Angle Section Stubs

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Abstract: Structural stainless steel requires appropriate recognition of its beneficial properties such as material nonlinearity and significant strain hardening. The recently proposed Continuous Strength Method (CSM) exploits those benefits through a strain based approach for both stocky and slender cross-sections, and is shown to yield a higher level of accuracy and consistency, as well as design efficiency, in the capacity predictions of stainless steel cross-sections. Although there have been extensive and comprehensive studies on SHS, RHS, round tubes and H-sections stubs, but research into cold-formed stainless steel equal-leg angle section stubs remains scarce. In this paper, the scope of the CSM is extended to cover the design of cold-formed stainless steel equal-leg angle section stubs. Developed FE models included material nonlinearities as well as initial geometric imperfections. A comprehensive parametric study has been carried out covering a wide range of slenderness with different cross section geometries for the considered angle stubs. Cross-section resistances obtained from the numerical study were used to assess the performance of the current Continuous Strength Method (DSM) guidelines and EC3 when applied for stainless steel equal-leg angle section stubs; obtained comparisons showed considerable conservatism. A modified design method for cold-formed stainless steel equal-leg angle section stubs is proposed herein following CSM techniques, which provides considerably more accurate predictions for the considered cold-formed stubs. Reliability of the proposed design equations is also presented showing a good agreement with both experimentally and numerically obtained results.

Keywords: Stainless Steel, Cold-formed, Equal-leg Angle, The Continuous Strength Method, Cross Section Strength

1

2

2*

1

2

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(CSM)
H

EC3

FE

1

				CSM CSM		CSM	
			-	CSM			
SEI/ASCE-8[1] 0.673 3(EC3)[3]	AS/NZS4673[2] CECS-410[4]		4	2	/	[2] [3]	[1] [4]
				2.1		(EC3)	
						EN1993-1-1[13]	EC3[3]
		(CSM)					
Nethercot[5,6] Ashraf[7]		Gardner Gardner	(b/t)				(
		Ashraf	$\varepsilon = \sqrt{(235 / \sigma_{0.2}) / (E / 210000)}$				(
[8]		“ “ ” ”	0.2	($\sigma_{0.2}$)			(σ_y)
	Gardner[9]	Afshan Gardner[10]		1-3 4			1-3
		CSM					/
		($\bar{\lambda}_p \leq 0.68$)					4
		CSM					
		Ahmed Ashraf[11]					3
							$\sigma_{0.2}$
							$N_{c,Rd}$
	CSM	CSM		A_g		A_{eff}	(1) (2)
							γ_{M0}
				1 2 3			
(SHS) H		(RHS)					
							$N_{c,Rd} = \frac{A_g \sigma_{0.2}}{\gamma_{M0}}$
		Zhang					(1)
Tan[12]				4			
							$N_{c,Rd} = \frac{A_{eff} \sigma_{0.2}}{\gamma_{M0}}$
							(2)

2.2 (CSM) CSM

2.2.1 (CSM)[14-16] [17-21] 2.2.3 CSM

2.2.2 CSM CSM 0.2 CSM

CSM “ ”

- CSM

$\bar{\lambda}_p$ ($\epsilon_{csm}/\epsilon_y$) CSM

(3) ϵ_{csm} $\bar{\lambda}_p > 0.68$

$\epsilon_y = \sigma_y / E$ (3) ($\epsilon_{csm}/\epsilon_y$) ϵ_{lb} CSM

EN1993-1-4[21] 15 ϵ_{lb} δ_u L

(DSM) (CSM)

$\bar{\lambda}_p$ $\bar{\lambda}_l$

(3) CSM

$\bar{\lambda}_p$ 0.68 [11]

$\frac{\epsilon_{csm}}{\epsilon_y} = \frac{0.25}{\bar{\lambda}_p^{3.6}} \leq \min\left(15, \frac{C_1 \epsilon_u}{\epsilon_y}\right)$ $\bar{\lambda}_p \leq 0.68$ (3) CSM

CSM $\epsilon_{lb,e} = N_{u,test} / (EA)$ $N_{u,test}$ $\epsilon_{lb,e}$

C_2 C_3 C_4) E_{sh} (4) (C_1 ϵ_y)

(3)) CSM C_1 (7) $N_y = A\sigma_y$

CSM C_2 (4) [24]

E_{sh} $\epsilon_u = C_3(1 - \sigma_y/\sigma_u) + C_4$ 0.2 CSM

σ_u $\epsilon_{csm}/\epsilon_y = \epsilon_{lb,e}/\epsilon_y$

$E_{sh} = \frac{\sigma_u - \sigma_y}{C_2 \epsilon_u - \epsilon_y}$ (4) $\epsilon_{csm}/\epsilon_y = \epsilon_{lb,e}/\epsilon_y = N_{u,test}/N_y$ (7)

ϵ_{csm} E_{sh} CSM $\bar{\lambda}_p > 0.68$

CSM σ_{csm} (5) () (0.68,1)

(6) $N_{csm,Rd}$ γ_{M0} CSM (8) $\bar{\lambda}_p = 0.68$

1.1

$\sigma_{csm} = \sigma_y + E_{sh}(\epsilon_{csm} - \epsilon_y)$ (5)

$N_{csm,Rd} = \frac{A\sigma_{csm}}{\gamma_{M0}}$ (6) $\frac{\epsilon_{csm}}{\epsilon_y} = \left(1 - \frac{0.222}{\bar{\lambda}_p^{1.050}}\right) \frac{1}{\bar{\lambda}_p^{1.050}}$ (8)

CSM () (9) [33-34]

$$N_{csm,Rd} = \frac{\epsilon_{csm}}{\epsilon_y} \frac{A\sigma_{0.2}}{\gamma_{M0}} \quad (9) \quad 3.5$$

3
3.1 ABAQUS[22] 1 FE

3.2 [23-24] S4R 2mm

		1		
		D&W	t/100	t/10
A50X6	A50X6-150	1.01	1.02	0.95
	A50X6-150R	1.01	1.03	0.94
A100X6	A100X6-300	0.93	0.98	0.89
	A100X6-300R	1.00	1.05	0.94
		0.99	1.02	0.93
		0.04	0.03	0.03

6 Arrayago [25] Rasmussen[26] A50-150 A100-300 FE -
 Ramberg-Osgood(R-O)[27] FE Dawson
 Walker [12]
 2 [28] A100×100×6 2

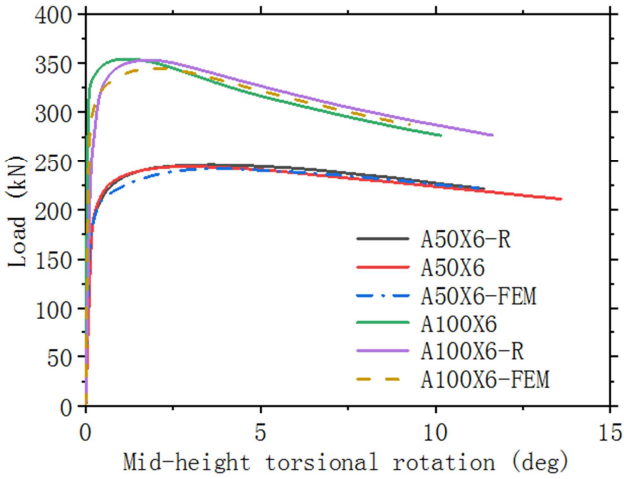
3.3 4
 470
 25-200mm 2-6mm (1.4420)
 0.25-3.0
 [12] A50×50×6

Dawson Walker 1) Gardner Nethercot[29] t
 [30] 2)t/100 3)t/10
 Riks [31-32] 2 Dawson Walker

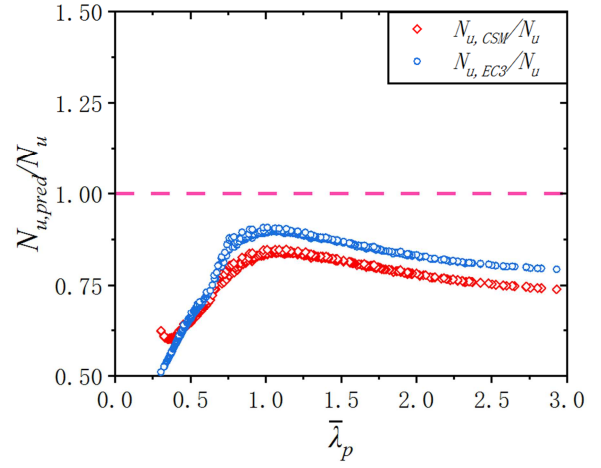
3.4 [12] (EC3)[3] CSM

2 [12]

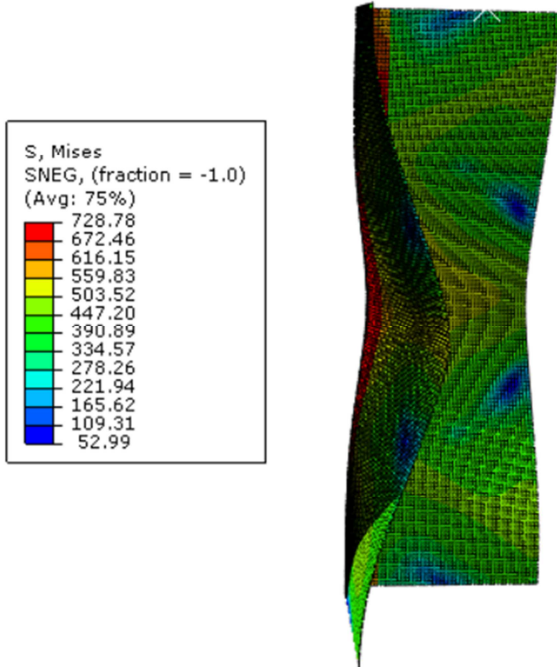
	E(GPa)	$\sigma_{0.2}$ (MPa)	σ_u (MPa)	R-O (n)
A50X6	198.2	278.2	637.0	6.8
	187.1	619.4	880.5	2.2



1 [12]



3 CSM



2 A100x100x6

6

CSM

$$L \quad (10) \quad \delta_u \quad 4$$

4

$$\epsilon_{csm} = \epsilon_{lb} - 0.002 = \frac{\delta_u}{L} - 0.002 \quad (10)$$

CSM

$$\text{CSM} \quad (3) \quad (8) \quad \text{CSM} \quad ($$

(11))

$$\epsilon_{csm} = \frac{0.0013}{\lambda_p^{-0.73}} \quad \frac{\epsilon_{csm}}{\epsilon_y} \leq \min \left(15, \frac{C_1 \epsilon_u}{\epsilon_y} \right) \quad (11)$$

$$\sigma_{csm} = E \epsilon_{csm} \quad (12)$$

$$N_{csm,Rd} = \frac{\sigma_{csm} A}{\gamma_{M0}} \quad (13)$$

$$(11) \quad (12) \quad \text{CSM} \quad (11)$$

$$\bar{\lambda}_p = 0.68$$

5

(EC3) CSM

N_u

$N_{u,pred}$
EC3

CSM

EC3

CSM

EC3

CSM

EC3

CoV

0.09

0.12

0.74

0.77

CSM EC3

$N_{u,pro}$
5

CSM

N_u

$N_{u,pro}/N_u$

1.01

0.03
(EC3)

CSM

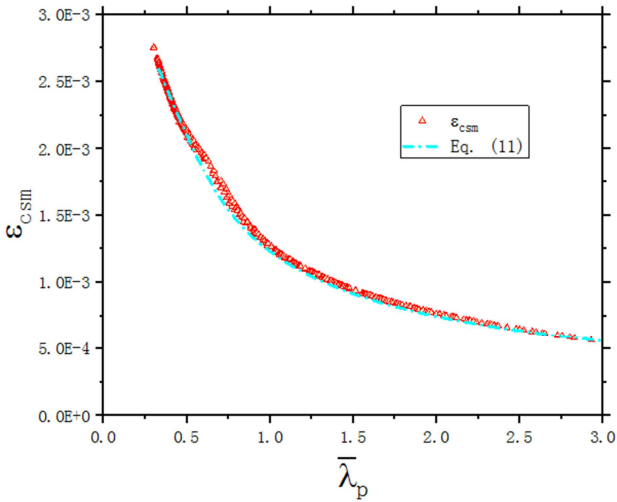
CSM

1.0

1.10
CSM
1.15

CSM

CSM



3

n	Kd,n	b	V _δ	V _r	γ _{M0}
470	3.04	1.01	0.06	0.05	1.12

8

FE
470

(EC3)

CSM

(EC3)

CSM

FE
CSM

CSM

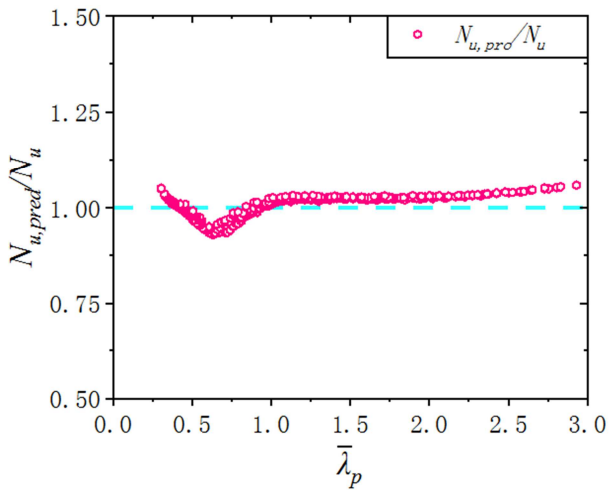
CSM

CSM

1.15
CSM

(2018JM5123)

4



5 CSM

FE

7

EN 1990 - Annex D[35]

CSM

3

n FE

k_{d,n}

n

()

b

V_δ

FE

V_r

γ_{M0}

[36]

1.3

0.06

0.05

CSM

γ_{M0}

EC3[3]

[1] SEI/ASCE8-02, Specification for the Design of Cold-Formed Stainless Steel Structural Members [Z], in, American Society of Civil Engineers (ASCE), Reston,2002.

[2] AS/NZS 4673, Cold-formed stainless steel structures [Z], in, Standards Australia,Sydney, 2001.

[3] EN 1993-1-4, Eurocode 3: Design of steel structures – Part 1.4:General rules –supplementary rules for stainless steels [Z], in, European Committee for Standardization(CEN), Brussels, 2006.

[4] CECS 410, Technical specification for stainless steel structures [Z], in, China plan press, Beijing, 2015.

[5] L. Gardner, D.A. Nethercot, Experiments on stainless steel hollow sections—Part 1: material and cross-sectional behaviour [J], J. Constr. Steel Res. 60 (2004) 1291–1318.

- [6] L. Gardner, D. Nethercot, Structural stainless steel design: a new approach [J], *Struct. Eng.* 82 (2004) 2–28.
- [7] L. Gardner, M. Ashraf, Structural design for non-linear metallic materials [J], *Eng. Struct.* 28 (2006) 926–934.
- [8] M. Ashraf, L. Gardner, D.A. Nethercot, Structural stainless steel design: resistance based on deformation capacity [J], *J. Struct. Eng.* 134 (2008) 402–411.
- [9] L. Gardner, The continuous strength method [C], *Proc. ICE - Struct. Build.* 161 (2008) 127–133.
- [10] S. Afshan, L. Gardner, The continuous strength method for structural stainless steel design [J], *Thin-Walled Struct.* 68 (2013) 42–49.
- [11] S. Ahmed, M. Ashraf, The Continuous Strength Method for slender stainless steel cross-sections [J], *Thin-Walled Struct.* 107 (2016) 362–376.
- [12] L Zhang, KH Tan, Experimental and numerical studies of fixed-ended cold-formed stainless steel equal-leg angle section columns [J], *Eng. Struct.* 184 (2019) 134–144.
- [13] EN 1993-1-1, Eurocode 3: Design of Steel Structures. Part 1-1: General Rules and Rules for Buildings [Z], Comité Europeo de Normalización, 2005.
- [14] M Ashraf, L Gardner, DA Nethercot. Structural stainless steel design: resistance based on deformation capacity [J]. *J. Struct. Eng.* 134 (2018) 402–411.
- [15] A Liew, L Gardner. Ultimate capacity of structural steel cross-sections under compression, bending and combined loading [J]. *Structures* 1 (2015) 2–11.
- [16] C Buchanan, L Gardner, A Liew. The continuous strength method for the design of circular hollow sections [J]. *J. Constr. Steel Res.* 118 (2016) 207–216.
- [17] G Shu, B Zheng, X Shen. Experimental and theoretical study on the behaviour of cold-formed stainless steel stub columns [J]. *Int. J. Steel Struct.* 13 (2013) 141–153.
- [18] O Zhao, B Rossi, L Gardner, B Young. Behaviour of structural stainless steel cross-sections under combined loading – part I: experimental study [J]. *Eng. Struct.* 89 (2015) 236–246.
- [19] O Zhao, B Rossi, L Gardner, B Young. Behaviour of structural stainless steel cross-sections under combined loading – part II: numerical modelling and design approach [J]. *Eng. Struct.* 89 (2015) 247–259.
- [20] O Zhao, B Rossi, L Gardner, B Young. Experimental and numerical studies of ferritic stainless steel tubular cross-sections under combined compression and bending [J]. *J. Struct. Eng.* 142 (2016) 40-51.
- [21] I Arrayago, E Real. Experimental study on ferritic stainless steel RHS and SHS cross-sectional resistance under combined loading [J]. *Structures.* 4 (2015) 69–79.
- [22] ABAQUS, ABAQUS User’s Manual v6.11, in, Dassault Systèmes Simulia Corp [M], Providence, RI, USA, 2011.
- [23] L. Gardner, D. Nethercot, Numerical modeling of stainless steel structural components-a consistent approach [J], *J. Struct. Eng.* 130 (2004) 1586–1601.
- [24] M. Ashraf, L. Gardner, D.A. Nethercot, Finite element modelling of structural stainless steel cross-sections [J], *Thin-Walled Struct.* 44 (2006) 1048–1062.
- [25] W. Ramberg, W.R. Osgood, Description of stress-strain curves by three parameters [J], *Natl. Advis. Comm. Aeronaut.* (1943).
- [26] K.J.R. Rasmussen, Full-range stress–strain curves for stainless steel alloys [J], *J. Constr. Steel Res.* 59 (2003) 47–61.
- [27] I. Arrayago, E. Real, L. Gardner, Description of stress–strain curves for stainless steel alloys [J], *Mater. Des.* 87 (2015) 540–552.
- [28] M. Ashraf, L. Gardner, D.A. Nethercot, Strength enhancement of the corner regions of stainless steel cross-sections [J], *J. Constr. Steel Res.* 61 (2005) 37–52.
- [29] R.G. Dawson, A.C. Walker, Post-buckling of geometrically imperfect plates [J], *J. Struct. Div.* 98 (1972) 75–94.
- [30] H.X. Yuan, Y.Q. Wang, Y.J. Shi, L. Gardner, Residual stress distributions in welded stainless steel sections [J], *Thin-Walled Struct.* 79 (2014) 38–51.
- [31] R.B. Cruise, L. Gardner, Residual stress analysis of structural stainless steel sections [J], *J. Constr. Steel Res.* 64 (2008) 352–366.
- [32] M. Jandera, L. Gardner, J. Machacek, Residual stresses in cold-rolled stainless steel hollow sections [J], *J. Constr. Steel Res.* 64 (2008) 1255–1263.
- [33] L. Gardner, A New Approach to Structural Stainless Steel Design [D], Department Of Civil And Environment Engineering, Imperial College Of London, London, UK, 2002.
- [34] M. Ashraf, Structural Stainless Steel Design: Resistance Based on Deformation Capacity [D], Department Of Civil And Environment Engineering, Imperial College London, UK, 2006.
- [35] EN 1990, Eurocode: Basis of Structural Design [Z], European Committee For Standardization (CEN), Brussels, Belgium, 2002.
- [36] S. Afshan, P. Francis, N. Baddoo, L. Gardner, Reliability analysis of structural stainless steel design provisions [J], *J. Constr. Steel Res.* 114 (2015) 293–304.